

7.3 题解答

截面特性:

$$A = 2 \times 300 \times 12 + 376 \times 10 = 10960 \text{ mm}^2$$

$$I_x = \frac{1}{12} (300 \times 400^3 - 290 \times 376^3) = 315363416 \text{ mm}^4, \quad w_x = 1576817 \text{ mm}^3$$

$$I_y = \frac{1}{12} (376 \times 1000 + 2 \times 12 \times 300^3) = 54031333 \text{ mm}^4$$

$$i_x = \sqrt{\frac{I_x}{A}} = 169.6 \text{ mm}, \quad i_y = \sqrt{\frac{I_y}{A}} = 70.2 \text{ mm}$$

$$\lambda_x = \frac{12000}{169.6} = 70.75, \quad \lambda_y = \frac{6000}{70.2} = 85.47$$

$$\lambda_x \sqrt{\frac{f_y}{235}} = 70.75, \quad \lambda_y \sqrt{\frac{f_y}{235}} = 85.47$$

B 类截面, 查表得 $\varphi_x = 0.746, \varphi_y = 0.652$

所有计算均按照《钢结构设计规范》GB50017

(1) 强度计算

$$\frac{800000}{10960} + \frac{120000000}{1.05 \times 1576817 \times 215} = 73.1 + 72.5 = 145.6 (N / \text{mm}^2) < 215 (N / \text{mm}^2), \quad \text{强}$$

度满足。

(2) 平面内稳定计算, 采用稳定极限承载力准则

$$\beta_{mx} = 0.65 - 0.35 \frac{80}{120} = 0.417$$

$$N_{Ex} = \frac{\pi^2 \times 2.06 \times 10^5 \times 10960}{1.1 \times 70.75^2} = 4042.7 \text{ kN}$$

$$\frac{800000}{0.746 \times 10960} + \frac{0.417 \times 120000000}{1.05 \times 1576817 \times (1 - \frac{0.8 \times 800}{4042.7})} = 97.825 + 37.625 = 135.45 (N / \text{mm}^2) < 215 (N / \text{mm}^2)$$

, 平面内稳定满足

(3) 平面外稳定计算

右边段平面外稳定:

$$\beta_{tx} = 0.65 + \frac{20}{120} = 0.82, \quad \varphi_b = 1.07 - \frac{85.47^2}{44000} = 0.904$$

$$\frac{800000}{0.652 \times 10960} + \frac{0.82 \times 120000000}{0.904 \times 1576817} = 111.95 + 69.03 = 180.98 (N/mm^2) < 215 (N/mm^2)$$

，右边段平面外是稳定的。

左边段的端弯矩（80）比右边段端弯矩（120）小， β_{tx} 也比右边段小，

所以左边段平面外肯定也是稳定的。

7.4 题解答

截面特性：

$$A = 154.59 \times 2 = 309.18 \text{ cm}^2, \quad I_x = 2(1702.4 + 154.59 \times 90^2) = 2507762.8 \text{ cm}^4$$

$$w_x = \frac{2507762.8}{98.8} = 25382.2 \text{ cm}^3$$

$$i_x = \sqrt{\frac{I_x}{A}} = 90 \text{ cm}, \quad \lambda_x = \frac{2930}{90} = 32.6, \quad \lambda_{0x} = \sqrt{\lambda_x^2 + 27 \frac{309.18}{2 \times 24.373}} = 37.5$$

采用 B 曲线，查表得 $\varphi_x = 0.908$

(1) 强度要求

$$\frac{2500000}{30918} + \frac{M_x}{1.05 \times 25382200} \leq 205, \quad \text{算得 } M_x \leq 3308.5 \text{ kN.m}$$

(2) 平面内稳定

$$N_{Ex} = \frac{3.14^2 \times 206000 \times 30918}{1.1 \times 37.5^2} = 40595.45 \text{ kN}$$

$$\frac{2500000}{0.908 \times 30918} + \frac{M}{25382200(1 - 0.908 \frac{2500}{40595.45})} \leq 205, \quad \text{算得 } M_x \leq 2778 \text{ kN.m}$$

(3) 单肢稳定要求

受压分肢在弯矩平面内的长细比

$$\lambda_{1x} = \frac{180}{3.32} = 54.2$$

受压分肢在弯矩平面外的长细比

$$\lambda_{1y} = \frac{1820}{24.66} = 73.8$$

$\lambda_{1y} > \lambda_{1x}$ ，并且属于 B 类曲线，查表得轴心受压构件的稳定系数为

$$\varphi_{1y} = 0.727$$

受压单肢所能够承受的压力为：

$$N \leq 0.727 \times 15459 \times 205 = 2303.9 \text{ kN}$$

$$\frac{2500 \times 0.9 + M_x}{1.8} \leq 2303.9, \text{ 算得 } M_x \leq 1897.02 \text{ kN.m}$$

综上，取最大的 $M_x = 1897.02 \text{ kN.m}$